

## Attachment 3

Example calculation Excel base

### Support reactions from Staad calculations (steel frame)

Example calculation made with DL+INS+WIND(+X)

(See attachment Staad picture Sheet No. 13)

User inserts chosen load case from support reactions report here:

Load case: DL+INS+WIND(+X)

Node	FX (kN)	FY (kN)	FZ (kN)	MX (kNm)	MY (kNm)	MZ (kNm)
1	-100,503	-137,168	-16,268	-3,346	-0,102	4,428
2	-100,875	-145,233	16,593	-1,994	-0,001	5,121
3	-3,189	589,687	-0,182	-0,104	-0,001	7,289
4	-3,434	590,562	-0,144	-0,051	-0,001	7,946

Forces effecting the stack foundation

	FX (kN)	FY (kN)	FZ (kN)	MX (kNm)	MY (kNm)	MZ (kNm)
1	100,503	137,168	16,268	3,346	0,102	-4,428
2	100,875	145,233	-16,593	1,994	0,001	-5,121
3	3,189	-589,687	0,182	0,104	0,001	-7,289
4	3,434	-590,562	0,144	0,051	0,001	-7,946

Ground filling calculation is optional. Only calculated when needed.

#### Insert height of the foundation

h= 800 mm foundation height

1729,8 kN weight of the foundation

#### Ground filling weight: ( calculate if needed)

Height of the ground filling: 1000 mm

surface area of foundation under the ground = 1643,31 m<sup>2</sup>

density of the ground = 19 kN/m<sup>3</sup>

ground filling weight = 31222,89 kN

**insert 0, if ground filling not calculated!!!**

## Bearing pressure calculations

User inserts the preliminary dimensions here.

### Measures of the structure

Insert measures A and B

A= 9300 mm length of the foundation

B= 9300 mm length of the foundation

P= 33850,5 kN

Mxx= -5092 kNm moment in direction x

Myy= 418,776 kNm moment in direction y

ex=  $M_{yy}/P = 0,012$  m

ey=  $M_{xx}/P = -0,150$  m

**Note!! Axes not equal with STAAD!!!**

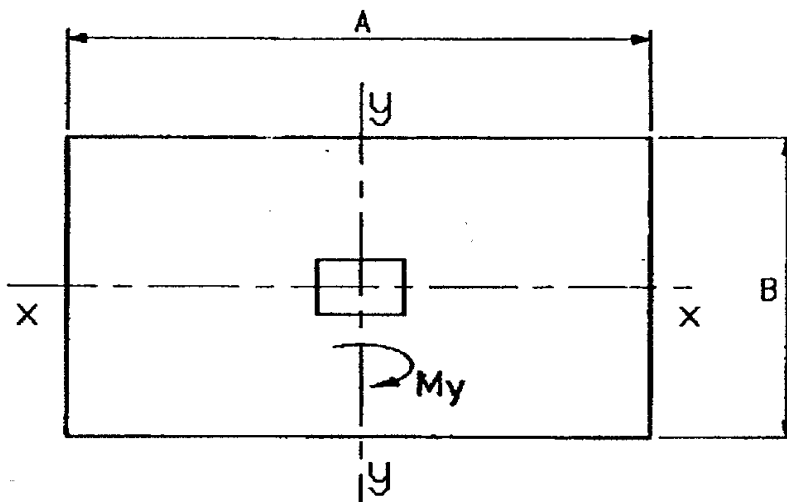


Figure 4. ( Reinforced concrete, Analysis and design, S.S. Roy)

After inserting the tables and measures, the base gives a zone as a result.  
 If the foundation is too small, calculation base commands to redesign the measures.

## Variables for x and y

res x = 0,012 place of resultant in direction x

res y = 0,150 place of resultant in direction y

LINE 1	resultant is inside Line 1	Overturing > 1.5
LINE 2	resultant is inside Line 2	Resultant is in Zone2
LINE 3	resultant is inside Line 3	Resultant is in Zone?
LINE 4	resultant is inside Line 4	Resultant is in Zone?
LINE 5	resultant is inside Line 5	Resultant is in Zone?

## Resultant is in Zone2

Based on the result of the zone, designer uses one of the following formula groups.

### Resultant in Zone 1 of base

If factor of safety for overturning is less than 1,5. Redesign size of base.

<u>Rectangular pad - uniaxial bending - no loss of contact</u>		
$p_1 = (P/A*B) + (6*M_{yy}/A^2*B)$	=	394,5 kN/m <sup>2</sup>
$p_2 = (P/A*B) - (6*M_{yy}/A^2*B)$	=	388,3 kN/m <sup>2</sup>
<u>Rectangular pad - uniaxial bending - loss of contact</u>		
$p_1 = 2*P / ((1,5*A) - (3*ex)) * B$	=	523,233 kN/m <sup>2</sup>
$x = 1,5*A - 3*ex$	=	13,91 m

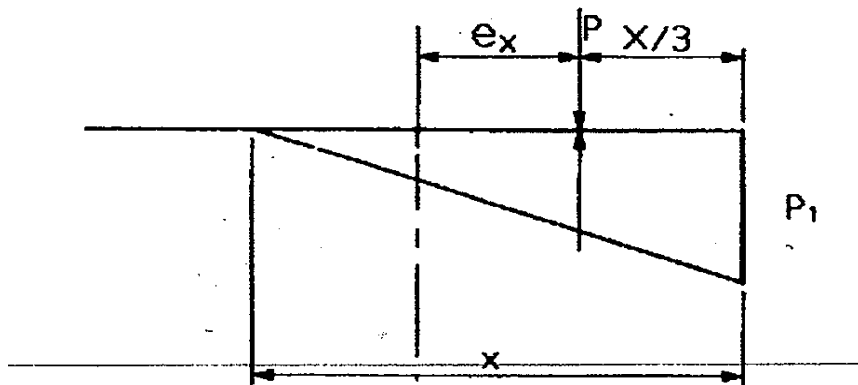


Figure 5. ( Reinforced concrete, Analysis and design, S.S. Roy)

**Resultant in Zone 2 of base**

$$\begin{aligned}
 p_1 &= (P/A \cdot B) + (6 \cdot M_{yy}) / (A^2 \cdot B) + (6 \cdot M_{xx}) / (A \cdot B^2) & = & 356,522 \text{ kN/m}^2 \\
 p_2 &= (P/A \cdot B) + (6 \cdot M_{yy}) / (A^2 \cdot B) - (6 \cdot M_{xx}) / (A \cdot B^2) & = & 432,488 \text{ kN/m}^2 \\
 p_3 &= (P/A \cdot B) - (6 \cdot M_{yy}) / (A^2 \cdot B) - (6 \cdot M_{xx}) / (A \cdot B^2) & = & 426,240 \text{ kN/m}^2 \\
 p_4 &= (P/A \cdot B) - (6 \cdot M_{yy}) / (A^2 \cdot B) + (6 \cdot M_{xx}) / (A \cdot B^2) & = & 350,274 \text{ kN/m}^2
 \end{aligned}$$

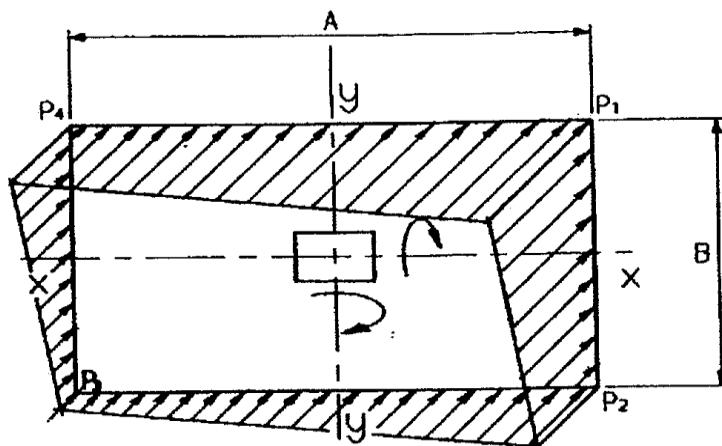


Figure 6. ( Reinforced concrete, Analysis and design, S.S. Roy)

### Resultant in Zone 3 of base

$$S = B/12 * (B/ey + (B^2/ey^2 - 12)^{1/2}) = 47,45 \text{ m}$$

$$\tan\alpha = (3 * (A - 2 * ex)) / (2 * (S + ey)) = 0,294$$

$$p1 = (12 * P / B * \tan\alpha) * (B + 2 * S / B^2 + 12 * S^2) = 570,86 \text{ kN/m}^2$$

$$p2 = (S - (B/2)) / (S + (B/2)) * p1 = 468,96 \text{ kN/m}^2$$

$$x1 = (S + (B/2)) * \tan\alpha = 15,3 \text{ m}$$

$$x2 = (S - (B/2)) * \tan\alpha = 12,6 \text{ m}$$

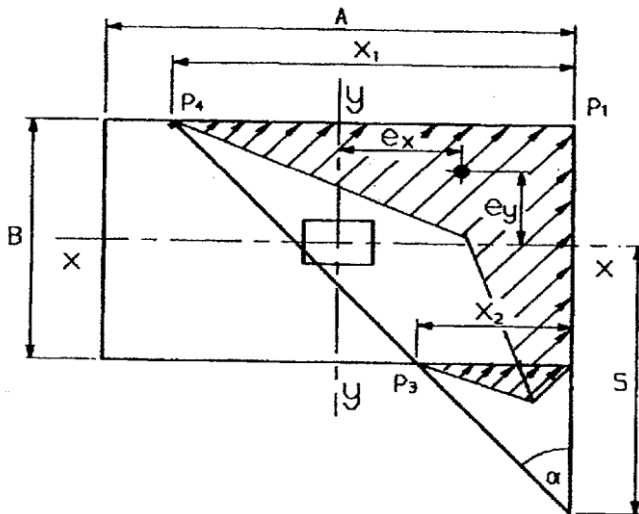


Figure 7. ( Reinforced concrete, Analysis and design, S.S. Roy)

### Resultant in Zone 4 of base

$$t = A/12 * (A/e_x + (A^2/e_x^2 - 12)^{1/2}) = 1165,19 \text{ m}$$

$$\tan\beta = 3(B - 2e_y) / (2(t + e_x)) = 0,012$$

$$p_1 = (12 * P / A * \tan\beta) * (A + 2t / A^2 + 12 * t^2) = 507,51 \text{ kN/m}^2$$

$$p_4 = (t - (A/2)) / (t + (A/2)) * p_1 = 503,48 \text{ kN/m}^2$$

$$y_1 = (t + (A/2)) * \tan\beta = 14,46 \text{ m}$$

$$y_2 = (t - (A/2)) * \tan\beta = 14,34 \text{ m}$$

$$p_2 = 0$$

$$p_3 = 0$$

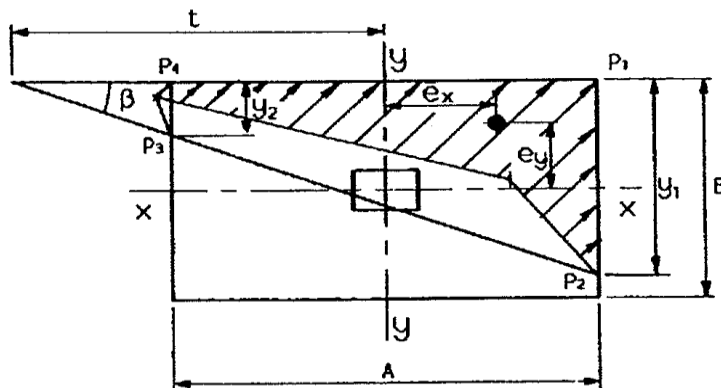


Figure 8. ( Reinforced concrete, Analysis and design, S.S. Roy)

### Resultant in Zone 5 of base

$$k = (e_x/A) + (e_y/B) = -0,015 \text{ m}$$

$$p_1 = (P/A \cdot B) \cdot k \cdot (12 - 3,9(6 \cdot k - 1) \cdot (1 - 2 \cdot k) \cdot (2,3 - 2 \cdot k)) = -128,91 \text{ kN/m}^2$$

$$p_2 = ((S - (B/2)) / (S + (B/2))) \cdot p_1 = -105,90 \text{ kN/m}^2$$

$$p_4 = (t - (A/2)) / (t + (A/2)) \cdot p_1 = -127,89 \text{ kN/m}^2$$

$$x_1 = (S - (B/2)) \cdot ((t + (A/2)) / (S + (B/2))) = 961,01 \text{ m}$$

$$y_1 = (t - (A/2)) \cdot ((S + (B/2)) / (t + (A/2))) = 51,68 \text{ m}$$

$$p_3 = 0$$

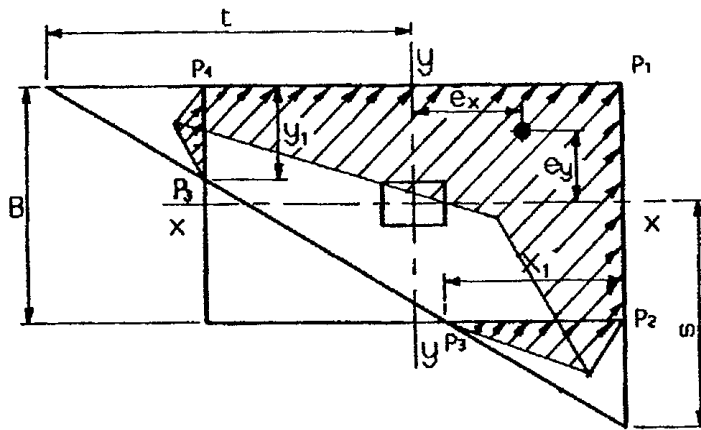


Figure 9. ( Reinforced concrete, Analysis and design, S.S. Roy)

### Zone areas in example foundation

Zone areas divided by straight lines, which has been given own equations

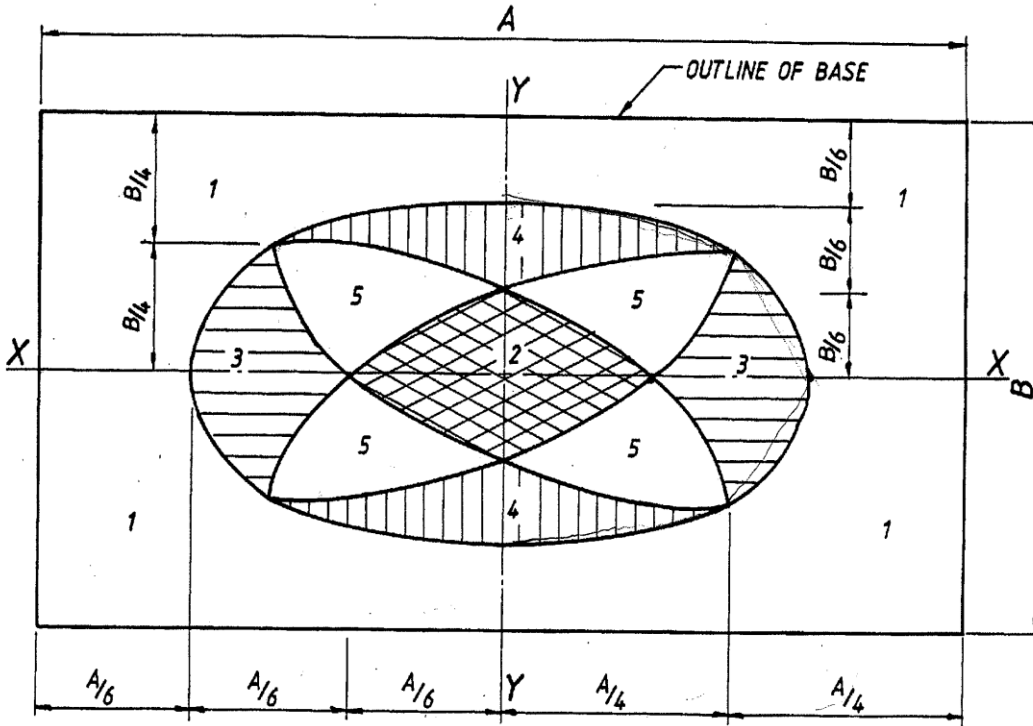


Figure 10. ( Reinforced concrete, Analysis and design, S.S. Roy)